

# *Design Meets Reality*

***Steve Allen***

Director

Strategic Transportation Investments Division

***Zane Pannell***

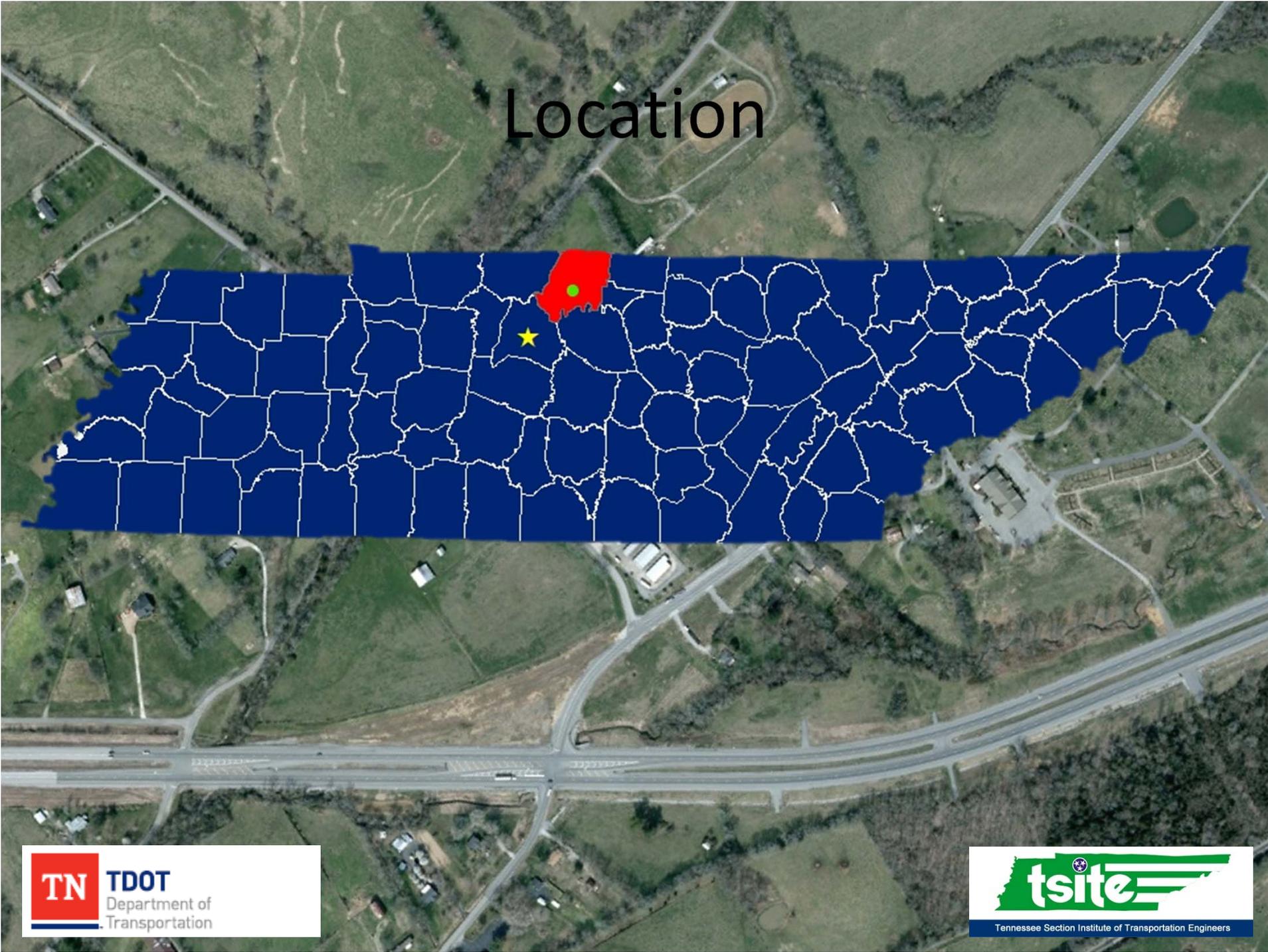
Transportation Project Specialist

Strategic Transportation Investments Division

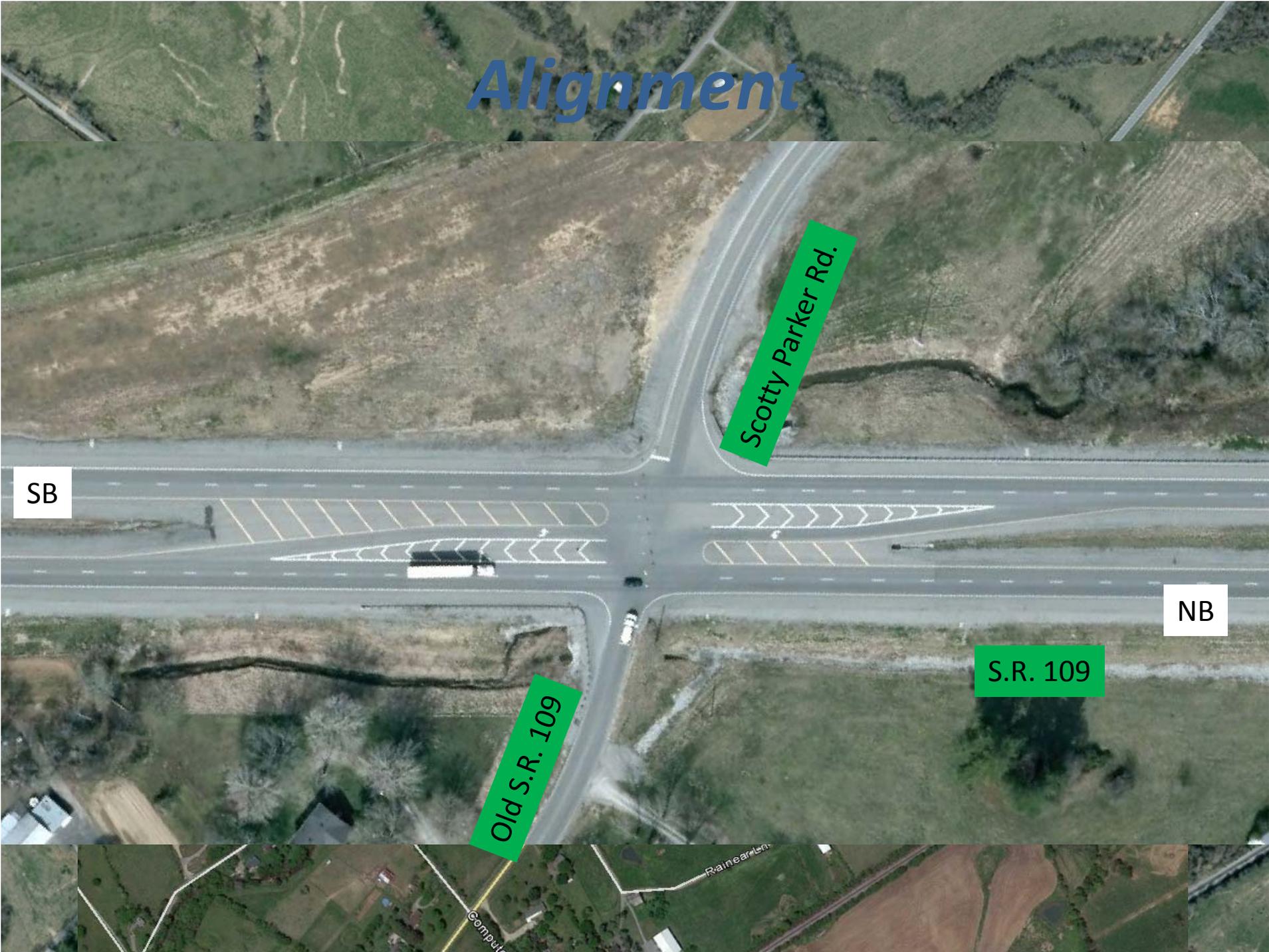
# Agenda

- Design Element Considerations
- Post Construction Issues
- Crash Statistics
- Proposed Concept
- Lessons Learned

# Location



# Alignment



Scotty Parker Rd.

SB

NB

S.R. 109

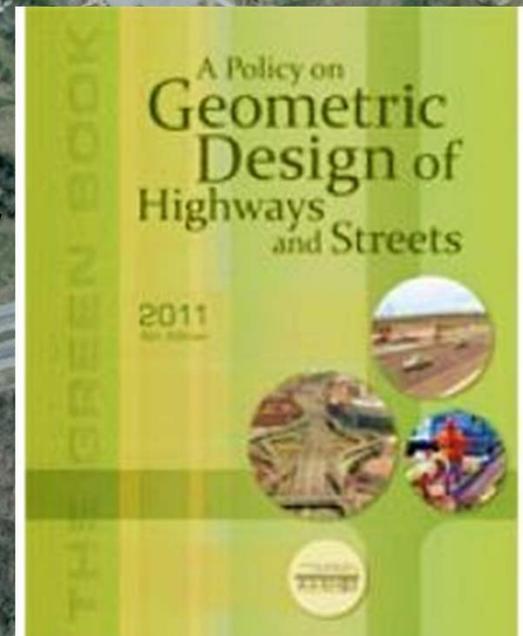
Old S.R. 109

RainearLn

Compu

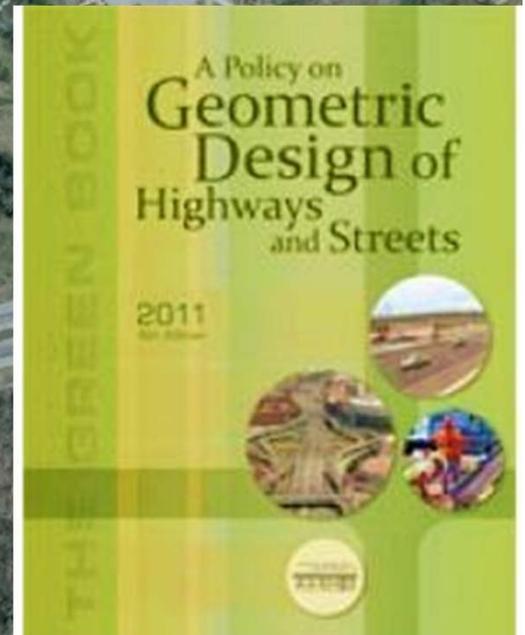
# 13 Controlling Elements of Design

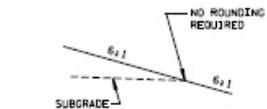
1. Design speed
2. Lane width
3. Shoulder width
- ~~4. Bridge width~~
5. Horizontal alignment
6. Superelevation
7. Vertical alignment
8. Grade
9. Stopping sight distance
- ~~10. Cross slope~~
- ~~11. Vertical clearance~~
- ~~12. Lateral offset to obstruction~~
- ~~13. Structural capacity~~



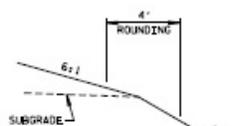
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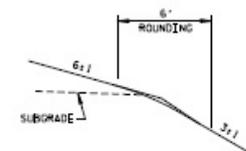




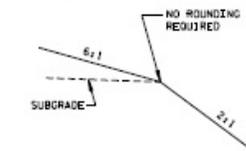
DETAIL A



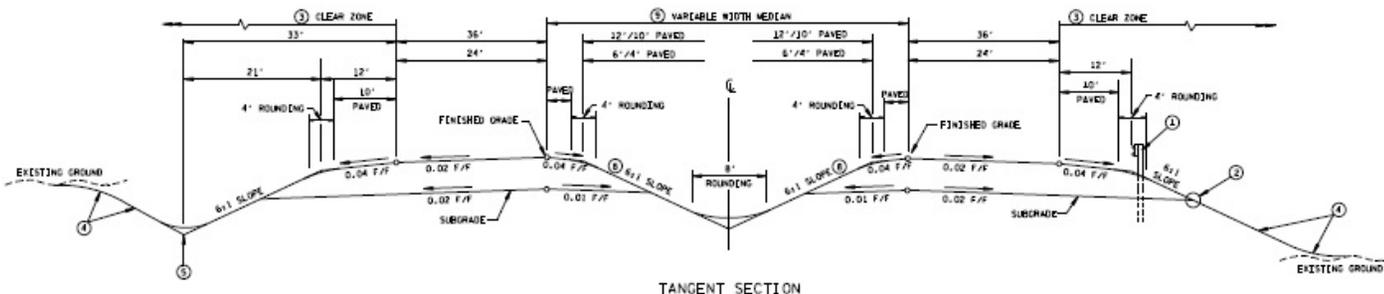
DETAIL B



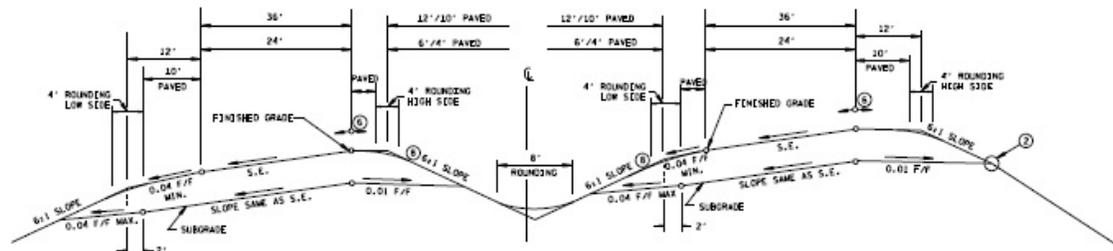
DETAIL C



DETAIL D



TANGENT SECTION



SUPERELEVATED SECTION

TABLE I. RECOMMENDED RURAL DESIGN SPEEDS (SEE PAGE 448) ①

TYPE OF TERRAIN	DESIGN SPEED (MPH)
LEVEL	70
ROLLING	60
MOUNTAINOUS	50

TABLE II. 4 AND 6 LANE ARTERIAL - DESIGN STANDARDS ②

DESIGN STANDARDS (FOR GIVEN DESIGN SPEED)	DESIGN SPEEDS (MPH)										
	30	35	40	45	50	55	60		65	70	
MINIMUM RADIUS (FT.) 0.04 MAX. S.E.	300	420	565	730	930	1190	1505			SEE PAGE 145	
MINIMUM RADIUS (FT.) 0.06 MAX. S.E.	275	380	510	660	835	1065	1340				
MINIMUM RADIUS (FT.) 0.08 MAX. S.E.	250	350	465	600	760	965	1205	1485	1820		
MAXIMUM RURAL GRADES %	LEVEL TERRAIN			5	5	4	4	3	3	SEE PAGE 450	
	ROLLING TERRAIN			6	6	5	5	4	4		
MAXIMUM URBAN GRADES %	LEVEL TERRAIN	8	7	7	6	6	5	5		SEE PAGE 476	
	ROLLING TERRAIN	9	8	8	7	7	6	6			
MINIMUM STOPPING SIGHT DISTANCE (FT.)	LEVEL TERRAIN	11	10	10	9	9	8	8		SEE PAGE 449	
	ROLLING TERRAIN	200	250	305	360	425	495	570	645		730
MINIMUM "K" VALUE	CREST VERTICAL CURVE	19	29	44	61	84	114	153	247	SEE PAGE 274	
	SAG VERTICAL CURVE	37	49	64	79	96	115	136	157		181
MINIMUM PASSING SIGHT DISTANCE (FT.)		1090	1280	1470	1625	1835	1985	2135	2285	2480	SEE PAGE 449
MINIMUM "K" VALUE FOR CREST VERTICAL CURVE		424	585	772	943	1203	1407	1628	1865	2197	SEE PAGE 276
SUPERELEVATION	SEE STANDARD DRAWINGS RD01-SE-2 AND RD01-SE-3.										

FOOTNOTES

- SEE DETAIL E FOR GUARDRAIL PLACEMENT AND GUARDRAIL STANDARD DRAWINGS (S-OR-SERIES).
- SEE DETAILS A, B, C, OR D FOR ROUNDING.
- CLEAR ZONE WIDTH SHALL BE DETERMINED FROM STANDARD DRAWING RD01-S-12. SEE THE "ROADSIDE DESIGN GUIDE," AASHTO, 2002, FOR FURTHER INFORMATION ON CLEAR ZONES.
- SEE STANDARD DRAWINGS RD01-S-11 AND RD01-S-11R FOR FILL AND CUT SLOPE TABLES, ROUNDING ON TOP OF CUT SLOPES AND THE OF FILL SLOPES, AND SPECIAL ROCK CUT TREATMENT.
- SEE STANDARD DRAWING RD01-S-11A FOR ROUNDING OF ROADSIDE DITCH SLOPES.
- THE SLOPES OF THE SHOULDER AND ROADWAY PAVEMENT SHALL NOT EXCEED AN ALGEBRAIC DIFFERENCE OF 0.07 FOOT PER FOOT.
- URBAN DESIGN SPEEDS ARE GENERALLY IN THE RANGE OF 30 TO 60 MILES PER HOUR (SEE PAGE 474).
- 6:1 SLOPES ARE DESIRABLE. SLOPES RANGING BETWEEN 6:1 AND 4:1 MAY BE USED UNDER SPECIFIC ADVERSE CONDITIONS SUCH AS TO FACILITATE DRAINAGE OR TO ESTABLISH A LEFT TURN LANE.
- 48 FEET MINIMUM, 64 FEET MINIMUM FOR A SIX LANE SECTION.
- ALTHOUGH THE SELECTED DESIGN SPEED ESTABLISHES THE LIMITING VALUES OF CURVE RADII AND MINIMUM SIGHT DISTANCE THAT SHOULD BE USED IN DESIGN, THERE SHOULD BE NO RESTRICTION ON THE USE OF FLATTER HORIZONTAL CURVES OR GREATER SIGHT DISTANCES WHERE SUCH IMPROVEMENTS CAN BE PROVIDED AS A PART OF AN ECONOMIC DESIGN (SEE PAGE 69).

GENERAL NOTES

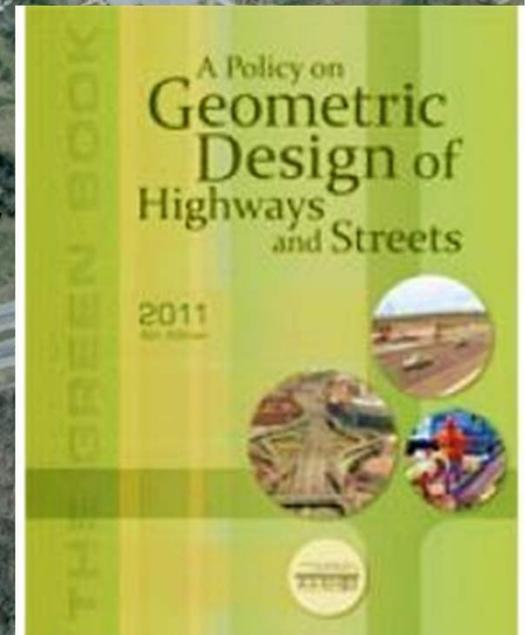
- SEE GUARDRAIL STANDARD DRAWINGS FOR TYPICAL GUARDRAIL PLACEMENT.
- PAGE NUMBERS REFERRED TO ON THIS DRAWING ARE FROM "A POLICY ON GEOMETRIC DESIGN OF HIGHWAYS AND STREETS," AASHTO, 2002, UNLESS OTHERWISE NOTED.
- REFERENCE SHOULD ALSO BE MADE TO THE "ROADSIDE DESIGN GUIDE," AASHTO, 2002.
- DESIRABLE HEIGHT-OF-WAY 35 SLOPE LINES PLUS FIFTEEN FEET TO TWENTY FEET.
- ALL NEW AND REHABILITATED BRIDGES SHALL BE DESIGNED FOR HS-20 LOADING. THE MINIMUM CLEAR WIDTH FOR NEW AND REHABILITATED BRIDGES SHALL BE EQUAL TO THE FULL WIDTH OF THE APPROACH ROADWAY, CURB-TO-CURB OR FULL SHOULDER WIDTH AS APPLICABLE.
- FOR EXISTING BRIDGES TO REMAIN IN PLACE, THEY SHOULD HAVE ADEQUATE STRUCTURAL STRENGTH AND A WIDTH AT LEAST EQUAL TO THE WIDTH OF THE TRAVELED WAY PLUS 2 FEET CLEARANCE ON EACH SIDE. BRIDGES SHOULD BE CONSIDERED FOR ULTIMATE WIDENING OR REPLACEMENT IF THEY DO NOT PROVIDE AT LEAST HS-20 LOADINGS. AS AN INTERIM MEASURE, NARROW BRIDGES SHOULD BE CONSIDERED FOR SPECIAL NARROW BRIDGE TREATMENTS SUCH AS SIGNING AND PAVEMENT MARKING.
- FOR ADDITIONAL URBAN DESIGN GUIDANCE AND CRITERIA, SEE PAGES 473-506.

MINOR REVISION -- FHWA APPROVAL NOT REQUIRED.

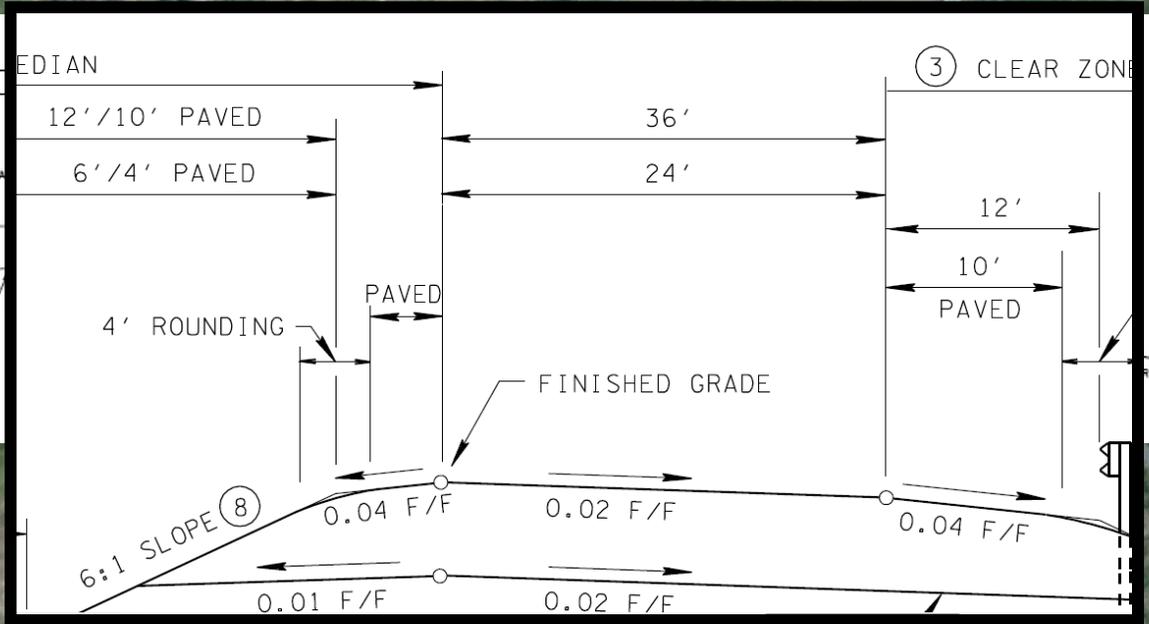
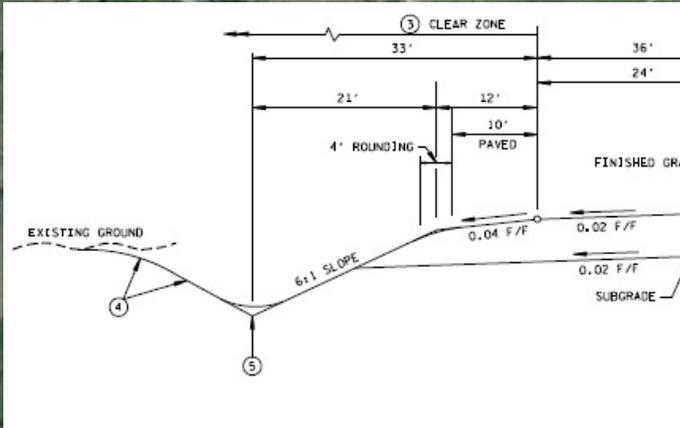
STATE OF TENNESSEE  
DEPARTMENT OF TRANSPORTATION  
DESIGN STANDARDS  
4 AND 6 LANE  
ARTERIAL  
HIGHWAYS WITH  
DEPRESSED MEDIANS  
10-15-02 RD01-TS-3A

# 13 Controlling Elements of Design

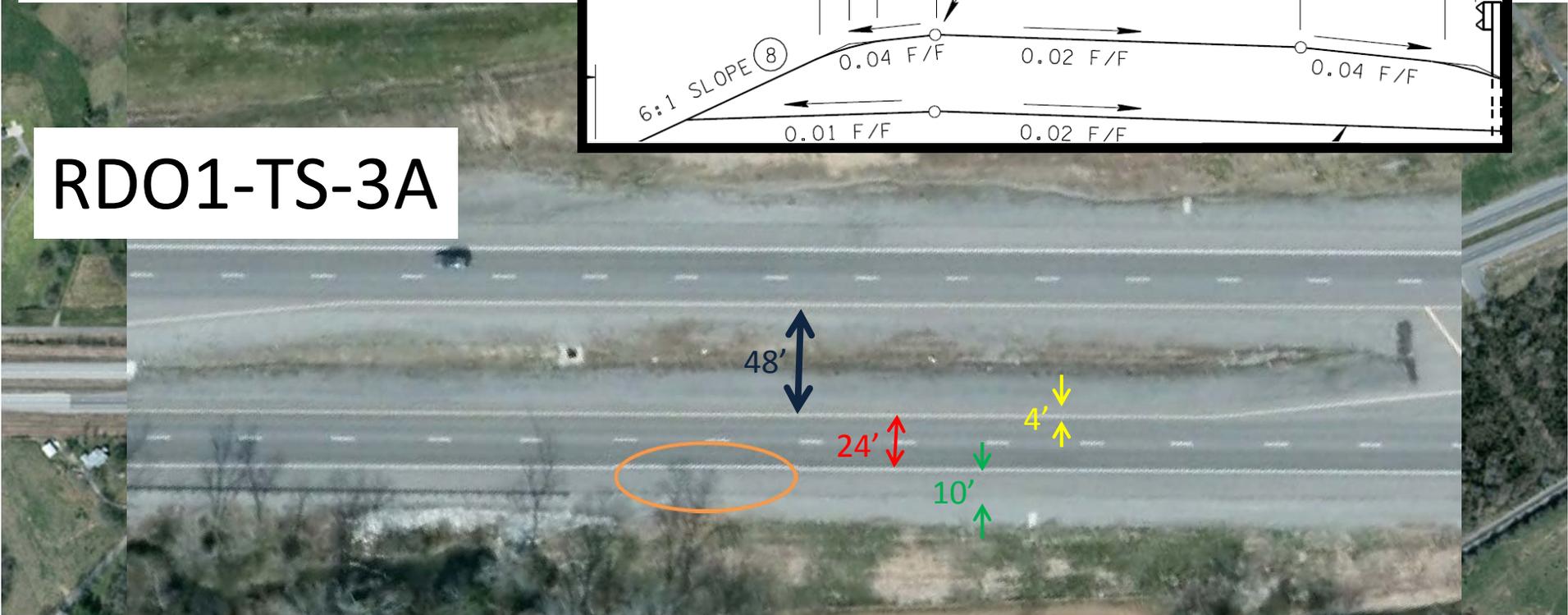
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# Lane Width and Shoulder Width

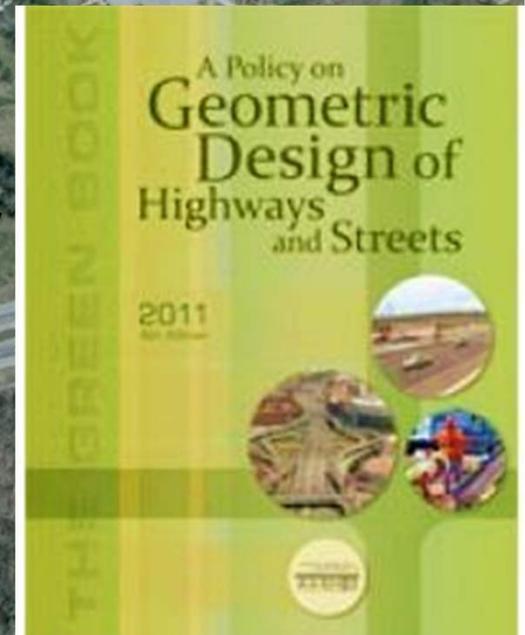


RDO1-TS-3A



# 13 Controlling Elements of Design

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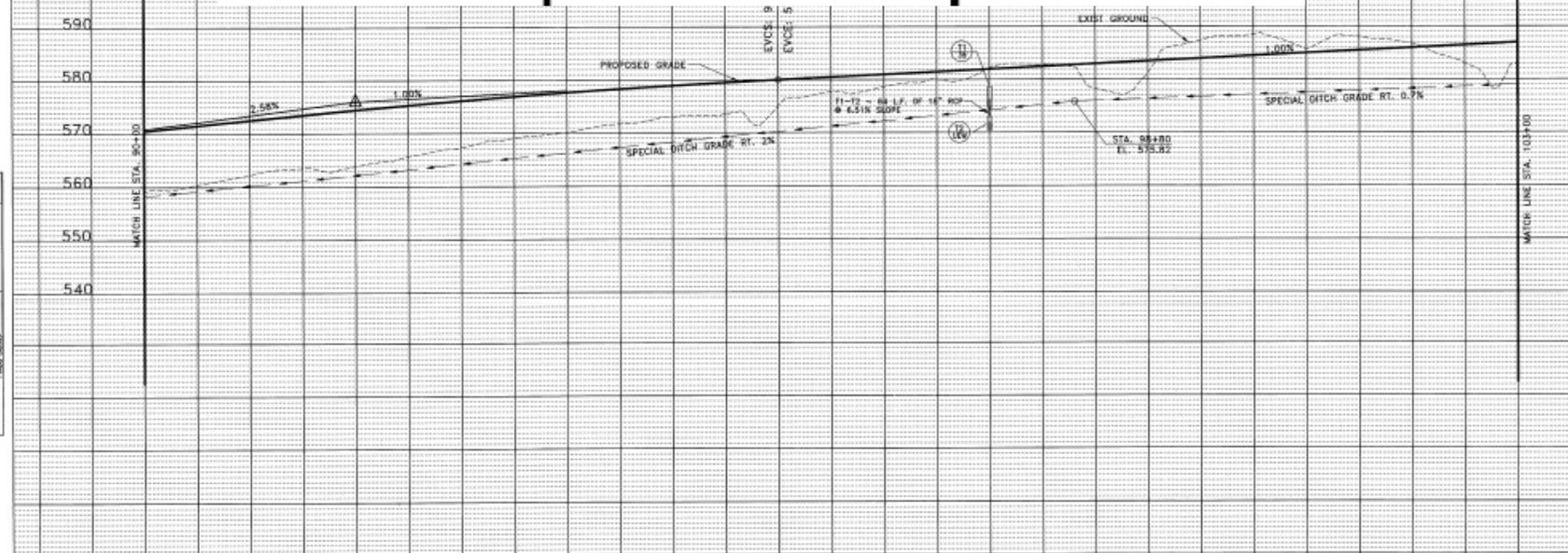
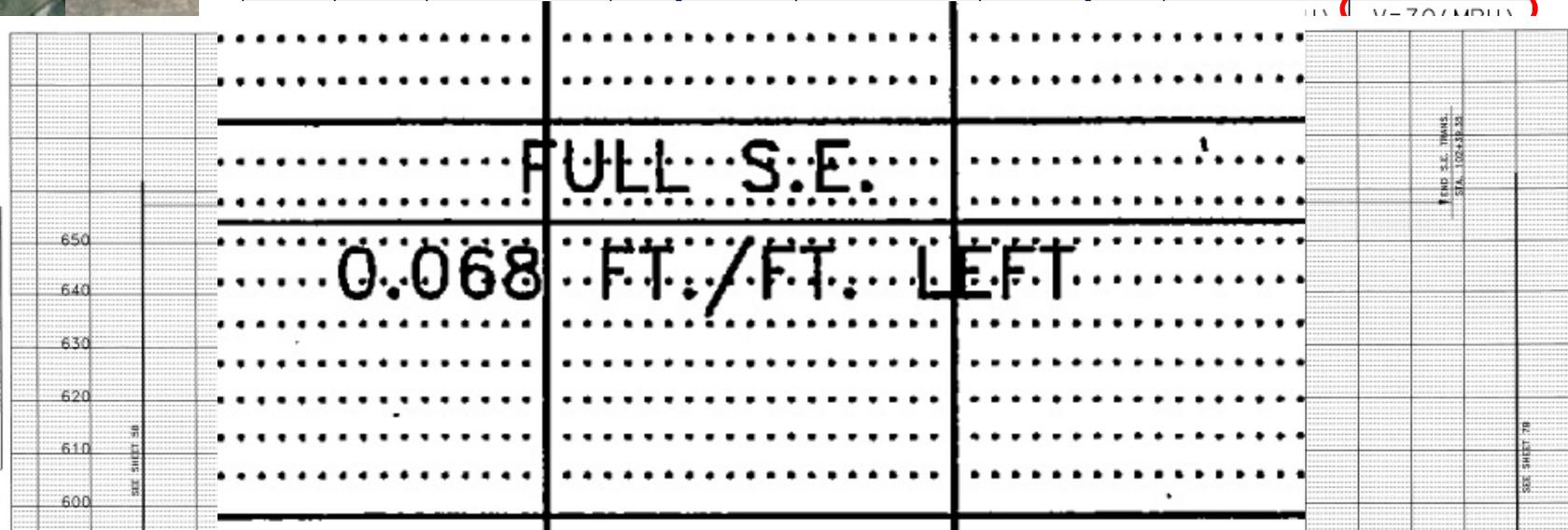
# Horizontal Alignment and Superelevation

E MAX=0.08 DESIRABLE

IN V-30 MP14

TYPE	YEAR	PROJECT NO.	SHEET NO.
R.O.W.	2003	SIP-109 (16)	8B
CONST.	2005	SIP-109 (16)	8B

REV. 04-11-05: REVISED CATCH BASIN STRUCTURE TYPE FROM 38 TO 39.

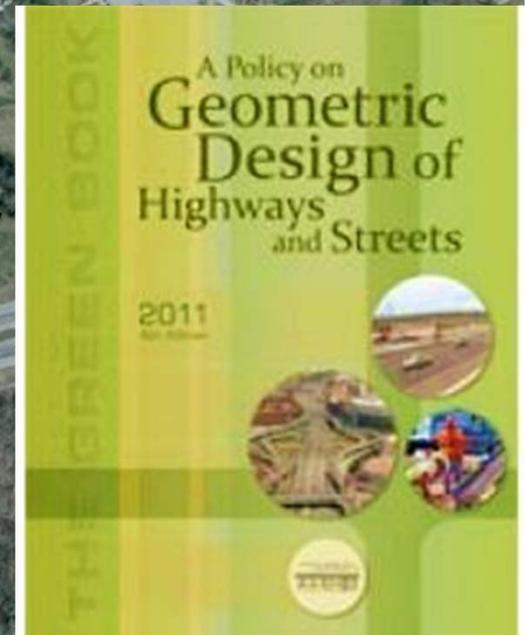


STATE OF TENNESSEE  
DEPARTMENT OF TRANSPORTATION  
BUREAU OF PLANNING & DEVELOPMENT

PROFILE  
SR-109

# 13 Controlling Elements of Design

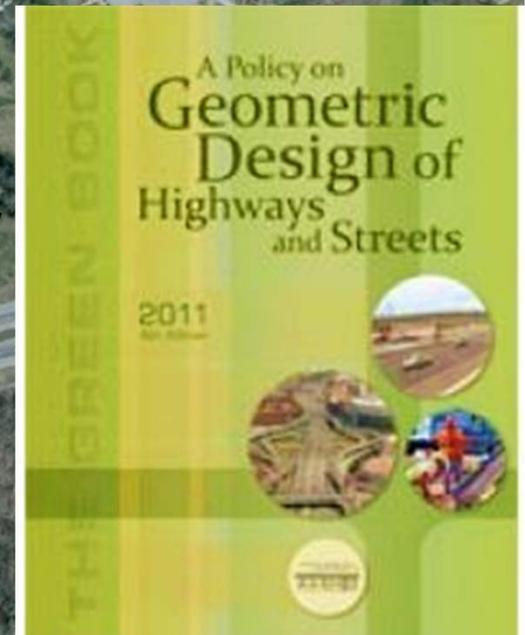
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# Stopping Sight Distance

RDO1-TS-3A

TABLE II. 4 AND 6 LANE ARTERIAL - DESIGN STANDARDS ⑩										
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		30	35	40	45	50	60	70	80	
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MINIMUM RADIUS (FT.) 0.06 MAX. S.E.		275	380	510	660	835	1065	1340	1620	SEE PAGE 145
MINIMUM RADIUS (FT.) 0.08 MAX. S.E.		250	350	465	600	760	965	1205	1485	SEE PAGE 145
MAXIMUM RURAL GRADES %	LEVEL TERRAIN			5	5	4	4	3	3	SEE PAGE 450
	ROLLING TERRAIN			6	6	5	5	4	4	SEE PAGE 450
	MOUNTAINOUS TERRAIN			8	7	7	6	5	5	SEE PAGE 450
MAXIMUM URBAN GRADES %	LEVEL TERRAIN	8	7	7	6	6	5	5	5	SEE PAGE 476
	ROLLING TERRAIN	9	8	8	7	7	6	5	5	SEE PAGE 476
MINIMUM "K" VALUE	SAG VERTICAL CURVE	11	10	10	9	9	8	8	7	SEE PAGE 274
	CREST VERTICAL CURVE	19	29	44	61	84	115	136	157	SEE PAGE 280
MINIMUM PASSING SIGHT DISTANCE (FT.)		1090	1280	1470	1625	1835	1985	2135	2285	SEE PAGE 449
MINIMUM "K" VALUE FOR CREST VERTICAL CURVE		424	585	772	943	1203	1407	1628	1865	SEE PAGE 276
SUPERELEVATION		SEE STANDARD DRAWINGS RDO1-SE-2 AND RDO1-SE-3.								

MINIMUM STOPPING SIGHT DISTANCE (FT.)

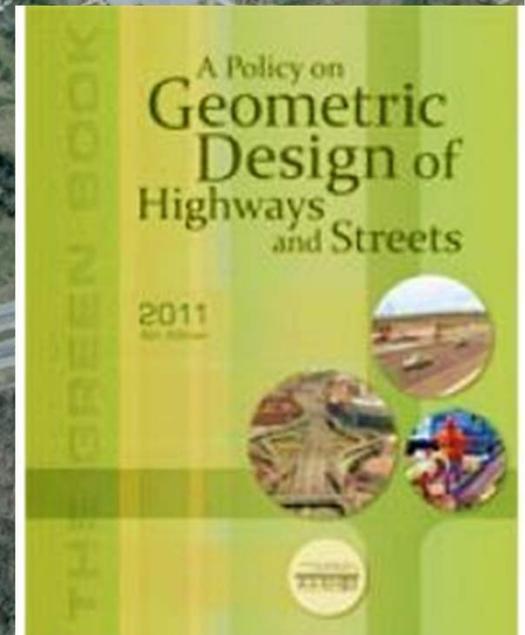
60

570



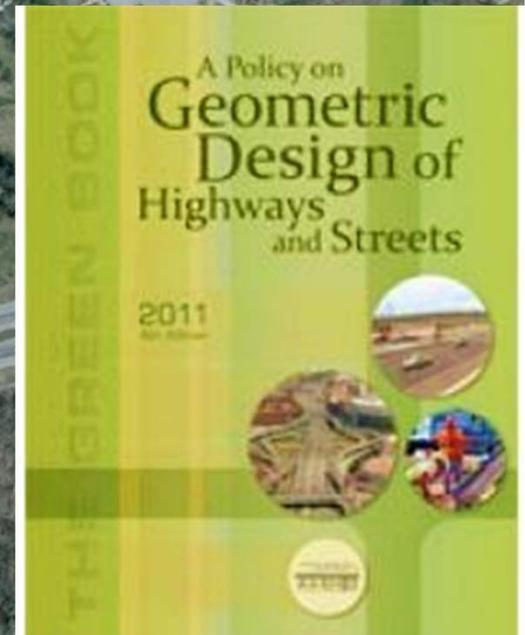
# Other Design Elements

- Intersection sight distance
- Offset turn lane
- Turn lane storage



# Other Design Elements

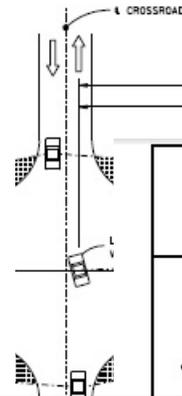
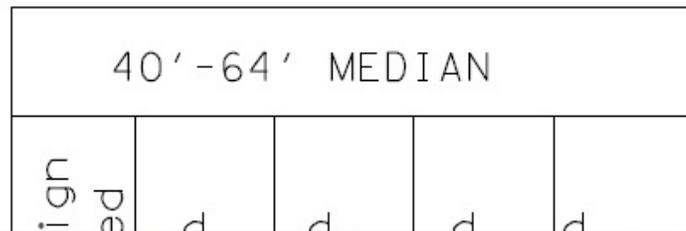
- Intersection sight distance
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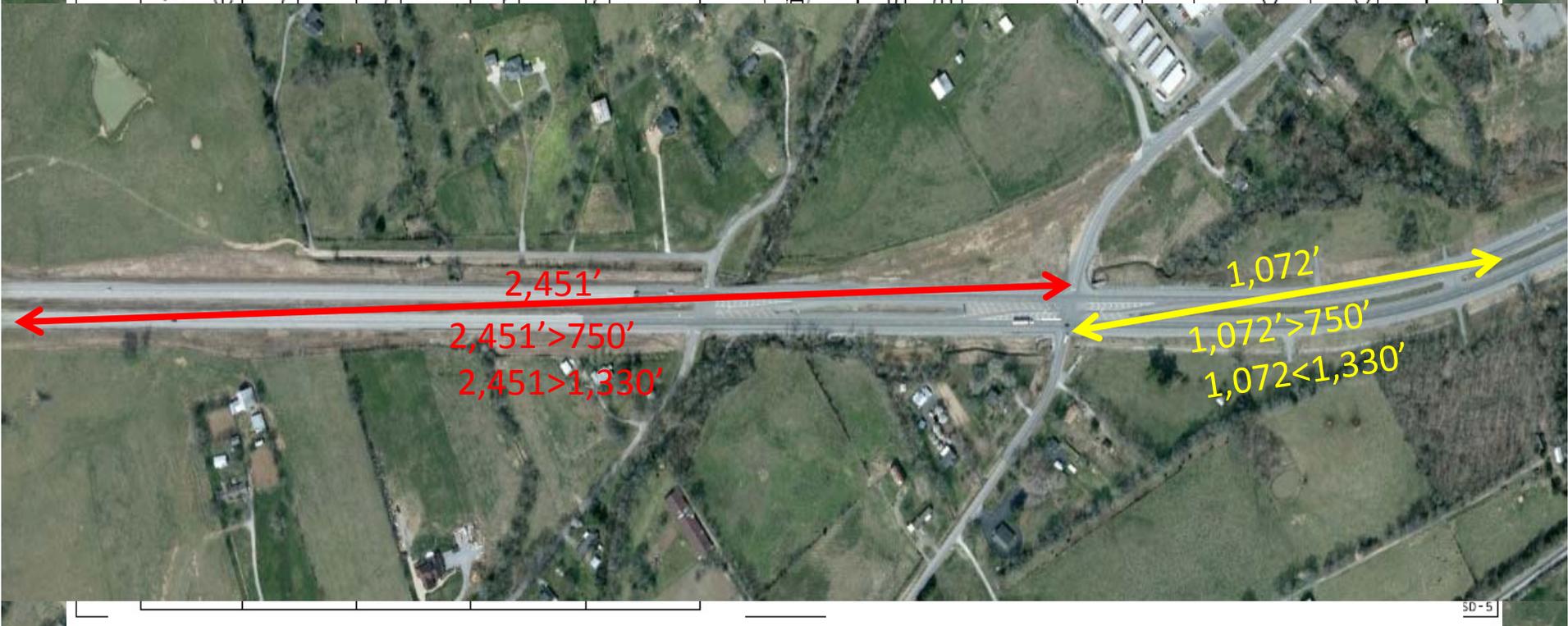
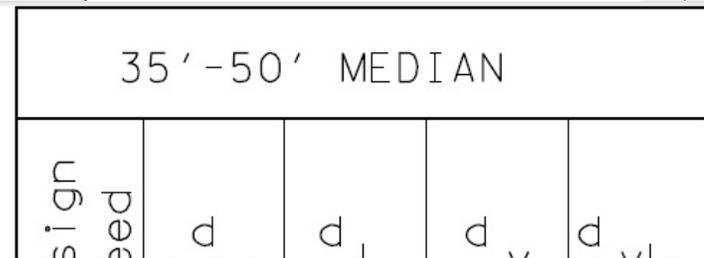
# Intersection Sight Distance

## RDO1-SD-5

Single Unit Truck

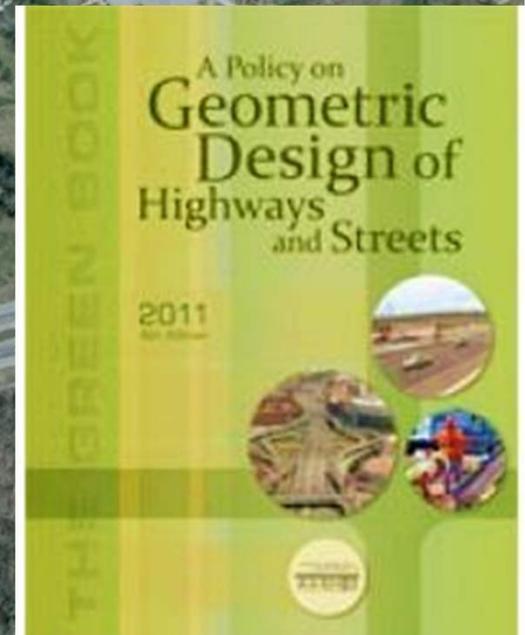


Intermediate Semi-Trailers



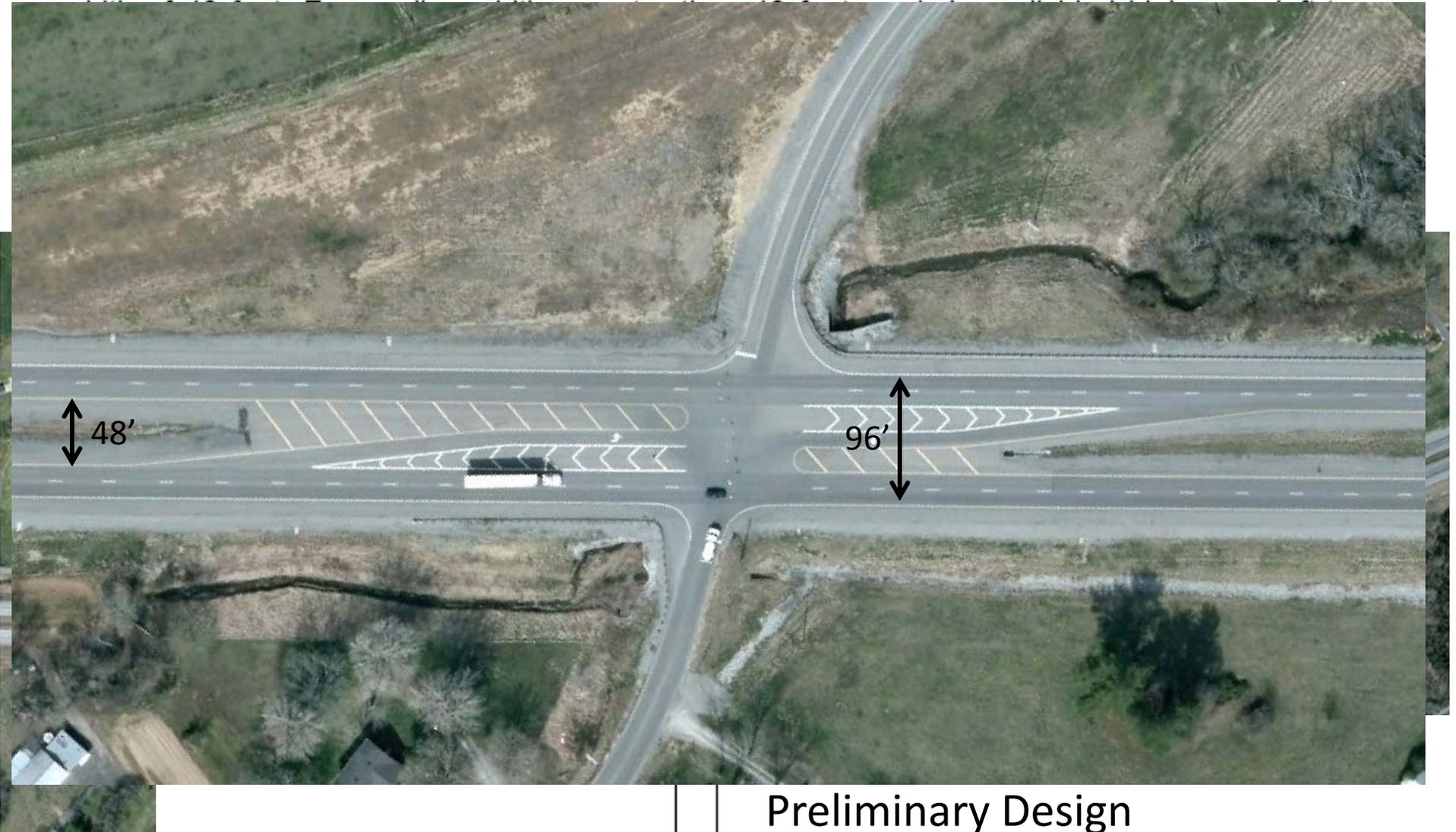
# Other Design Elements

- Intersection sight distance
- Offset turn lane
- Turn lane storage



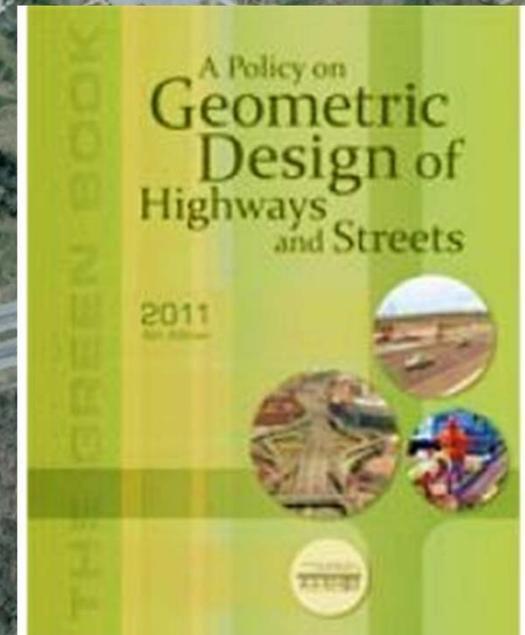
# Offset Turn Lane Design

The following guidelines apply to four-lane divided highways with a maximum median

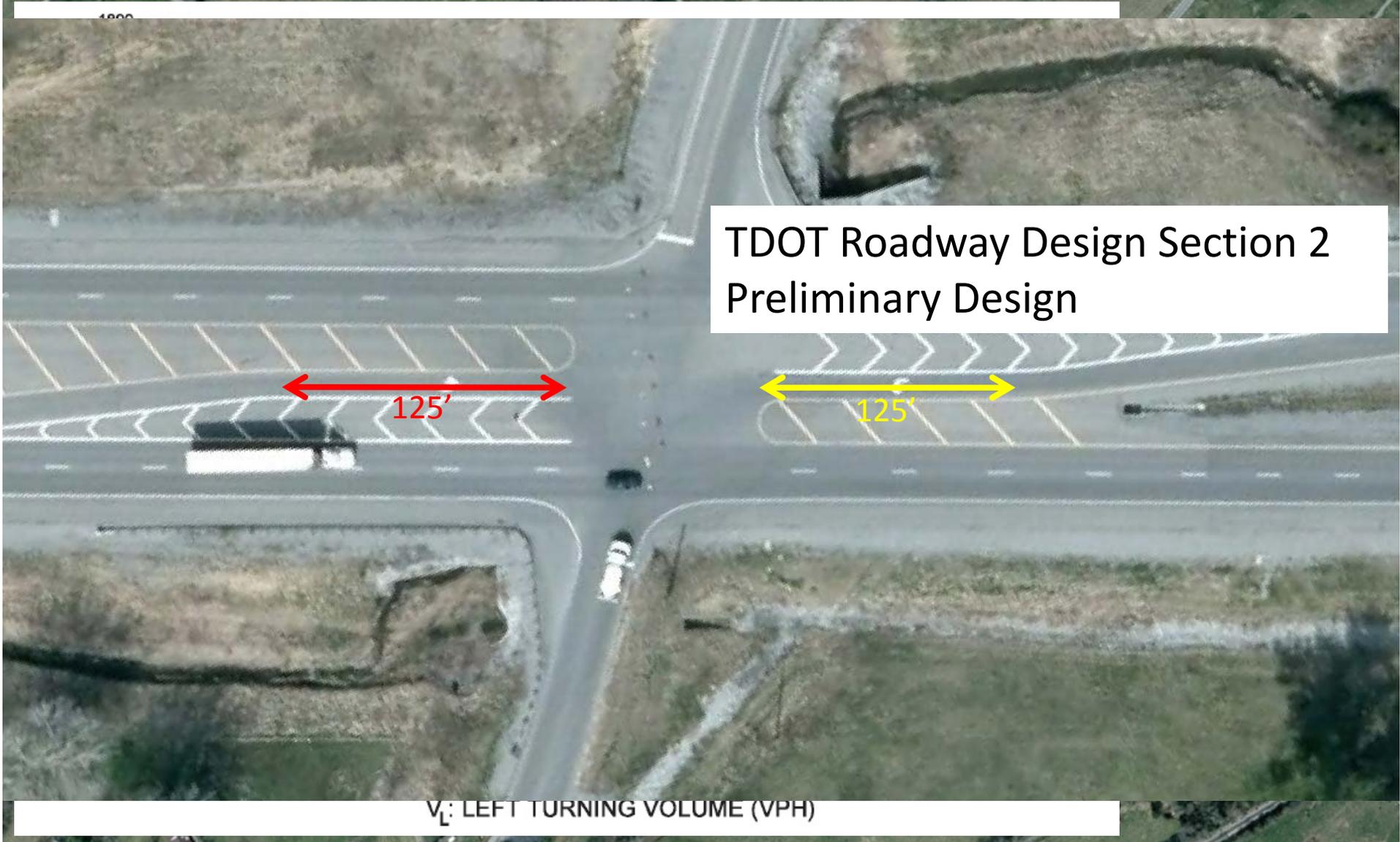


# Other Design Elements

- Intersection sight distance
- Offset turn lane
- Turn lane storage



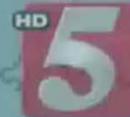
# Turn Lane Storage Design



# So, What's the Problem

**BREAKING NEWS**  
**SUMNER COUNTY**

★ **Nashville**





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## THP: 2 Portland High students killed in crash

Adam Tamburin, The Tennessean and Josh Cross, Gannett Tennessee 12:13 a.m. CST November 4, 2014



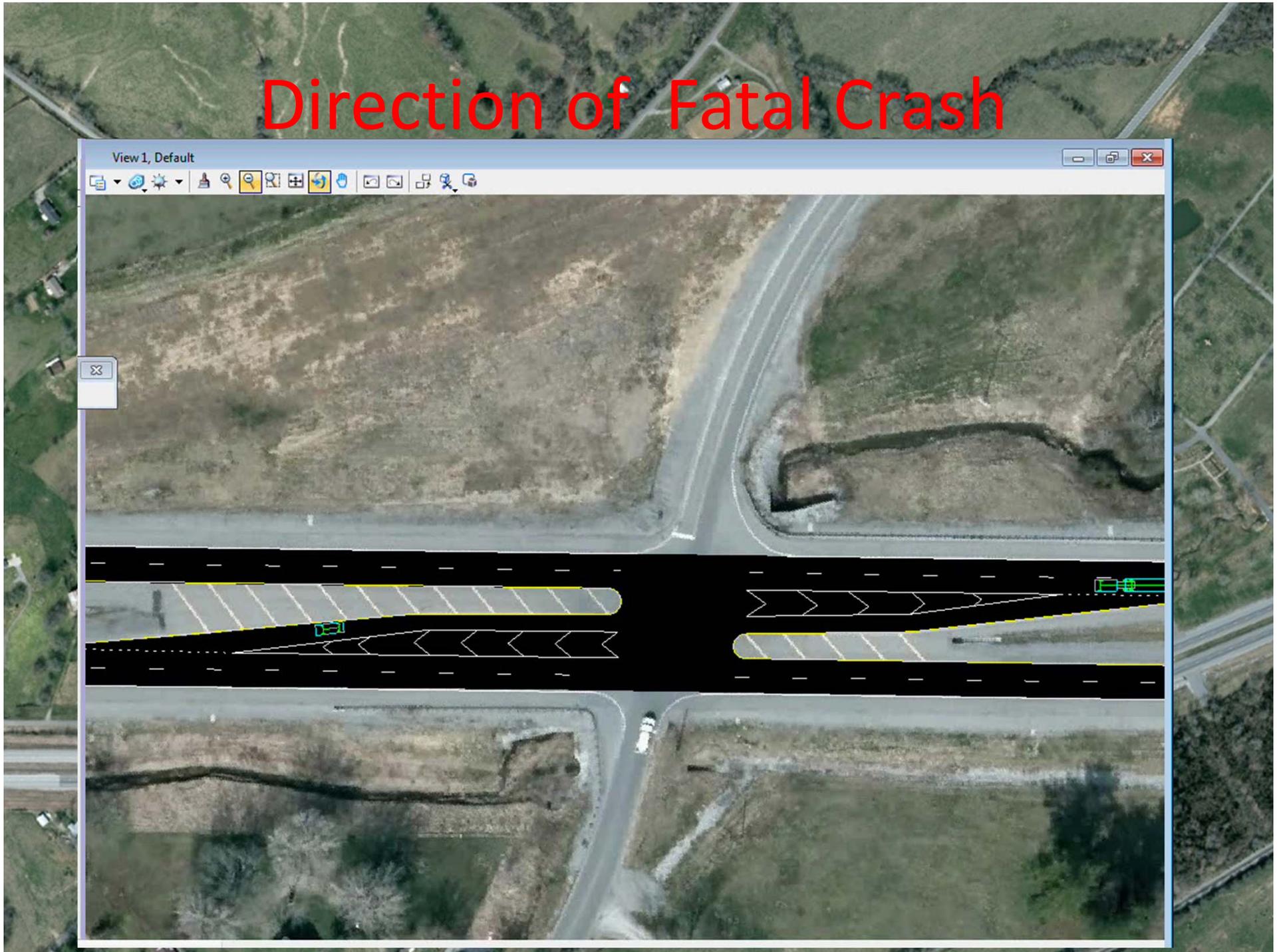
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3 MONTHS FOR  
THE PRICE OF 1

## 2 hurt in wreck Wednesday at site of fatal Nov. crash

Dessislava Yankova, Nashville 7:18 a.m. CST February 5, 2015

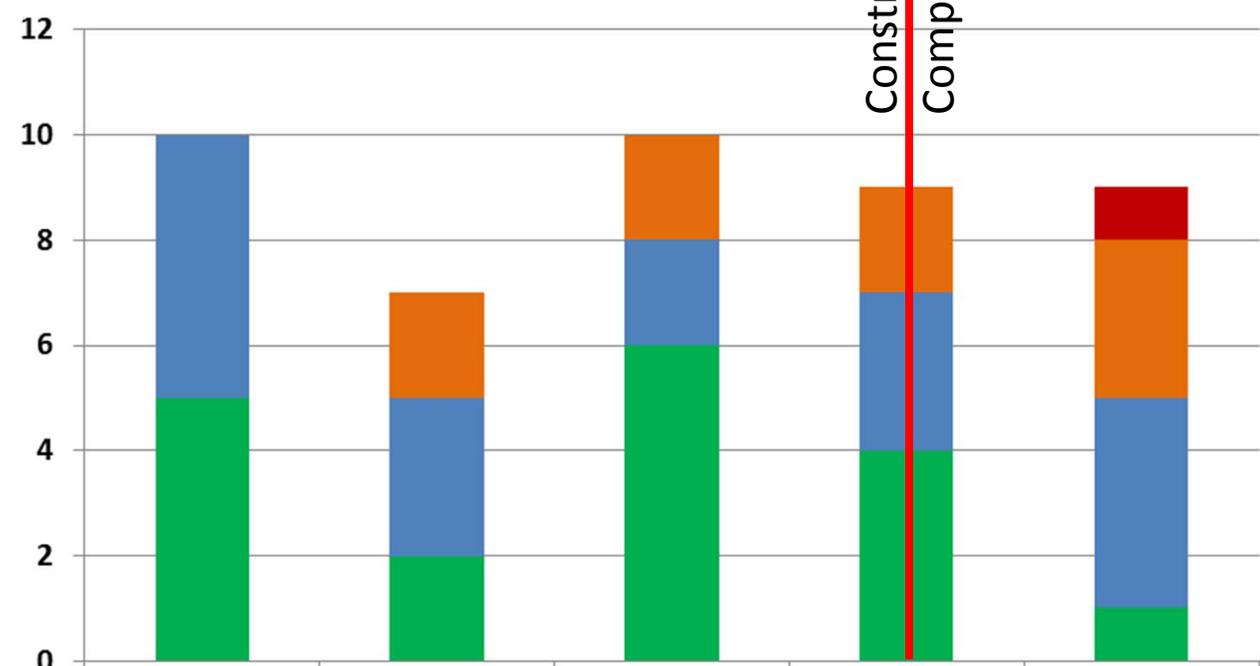


# Direction of Fatal Crash



# Total Crashes

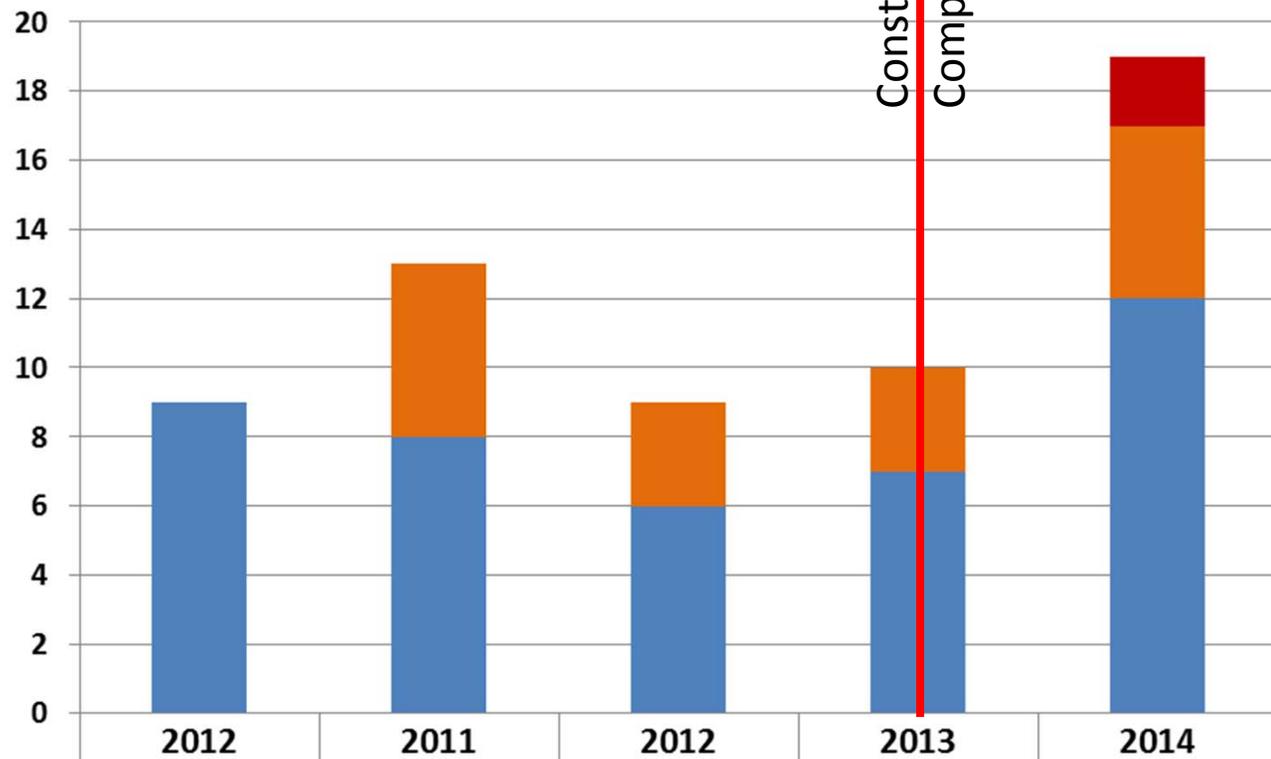
Construction  
Completed



	2010	2011	2012	2013	2014
<b>Fatal Crashes</b>	0	0	0	0	1
<b>Incapacitating Injury Crashes</b>	0	2	2	2	3
<b>Other Injury Crashes</b>	5	3	2	3	4
<b>Property Damage Crashes</b>	5	2	6	4	1

# Injury Severity

Construction  
Completed



Total Killed	0	0	0	0	2
Total Incapacitating Injuries	0	5	3	3	5
Total Other Injuries	9	8	6	7	12

# Clearance Time

$V_i = 0 \text{ mph}$   
 $V_f = 22 \text{ mph}$   
 $S = 125 \text{ ft}$   
 $a = ?$   
 $a = \frac{V_f^2 - V_i^2}{2S}$   
 $a = 4.12 \text{ ft/s}^2$

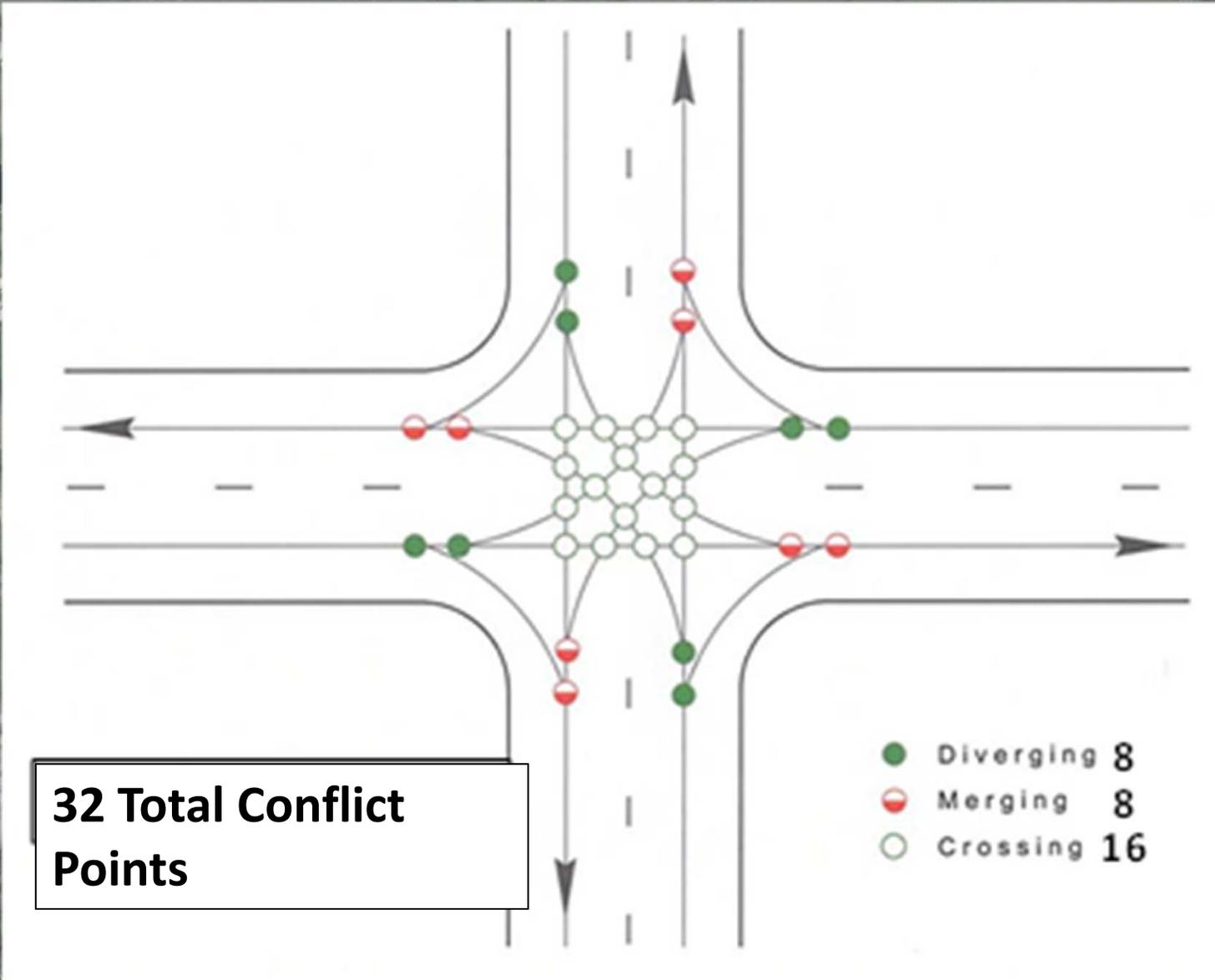
$V = 70 \text{ mph}$   
 $t = 7.83 \text{ sec}$   
 $s = ?$   
 $s = V * t$   
 $s = 804 \text{ ft}$

$s = 125 \text{ ft}$   
 $t = 7.83 \text{ sec}$

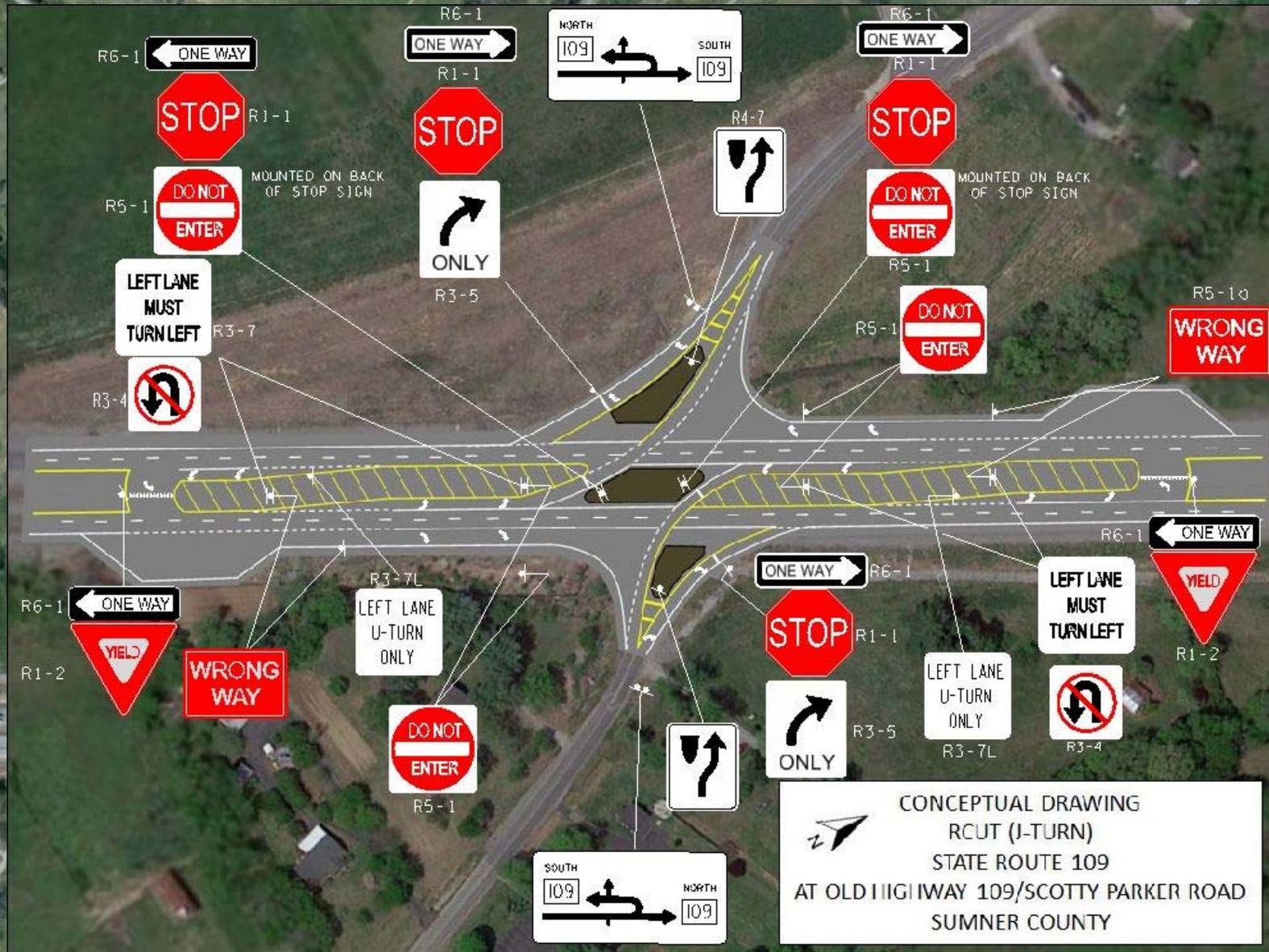
$V = 60 \text{ mph}$   
 $t = 7.83 \text{ sec}$   
 $s = ?$   
 $s = V * t$   
 $s = 689 \text{ ft}$

$V_i = 0 \text{ mph}$   
 $V_f = 22 \text{ mph}$   
 $a = 4.12 \text{ ft/s}^2$   
 $t = ?$   
 $t = \frac{V_f - V_i}{a}$

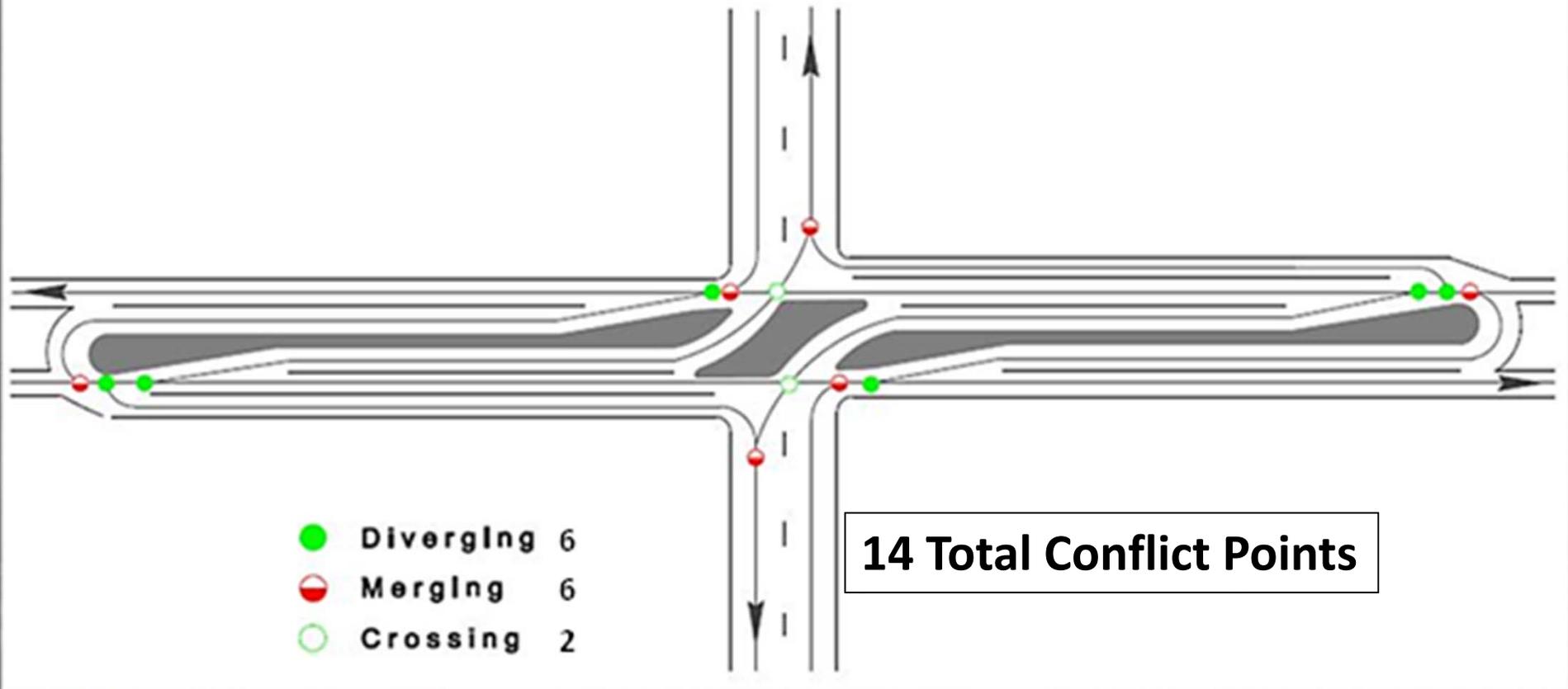
# Existing Conflicts



# Conceptual Design



# Conceptual Conflicts



# *Lessons Learned*

- Visualize design
- Safety considerations
- Future training
- Check all appropriate design vehicles



# *THANK YOU*

## *Questions & Comments*

# Contact Information

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Strategic Transportation Investments Division

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Email: [Zane.Pannell@Tn.Gov](mailto:Zane.Pannell@Tn.Gov)



# Conceptual Design

